Addendum Number 1
Date: 08/03/2023
Project: Interpretive Center Project
Owner: Trinidad Rancheria

This addendum provides changes and/or clarifications, to the Contract Documents. These modifications pertain to the sections referenced below and to all other referenced or applicable sections in the Contract Documents.

Please sign the addendum receipt acknowledgment form and return to the Owner with your cost proposal and other required forms and documents.

Changes and/or clarifications to the contract documents are as follows:

1. **What are the permits required, and what is the status of permitting?**

   The project will require both a Coastal Development Permit and a Building permit from the City of Trinidad. No work on the project can start until all permits are obtained by the Trinidad Rancheria. Permitting may take several months to complete. Permit conditions that effect work will need to be resolved by contract change order at a later date.

2. **Is equipment and fixtures shown on the project plans to be included in the project bid?**

   Yes: Items are to be included in Bid Items 36 - Kitchen Equipment, and 37 – Fixtures. See also descriptions in Bid Section 011000 and equipment schedule on Plan Sheet A3.

3. **What is the federal wage type determination for this project?**

   Building (Commercial)

4. **Is a soils report available for this project?**

   Yes – See attached

5. **Will the Security System be a part of this bid?**

   No – Security company will be contracted directly from the Trinidad Rancheria. Contractor shall coordinate with the security company for installation of security system and equipment.

Addendum Receipt Acknowledgement Form

Receipt of Acknowledgement:

My firm received Addendum No. _____, consisting of _____ pages, for the

_____

Project on _____, 20___.

Name of Firm _____
Commercial)
Name (Print) _____

Name (Signature) _____

Date: _____

END OF SECTION 009100

TRINITY VALLEY CONSULTING ENGINEERS, INC
Engineering – Surveying – Land Planning – Construction Management

R2 Soils Report

For

TRINIDAD PIER INTERPRETIVE CENTER
1 BAY STREET
TRINIDAD, CALIFORNIA
APN: 042-071-012

Report Provided For:
TRINIDAD RANCHERIA TRANSPORATION DEPARTMENT
Post Office Box 630
Trinidad CA 95570

Report Provided By:
Trinity Valley Consulting Engineers, Inc.
67 Walnut Way / Post Office Box 1567
Willow Creek, California 95573
(530) 629-3000 Fax: (530) 629-3011

May 2020
Project Number: 427.11

Table of Contents:

Attachments:

1. Location Map
2. Assessor Parcel Map/Humboldt County GIS Map & Data
3. Taber Consultants Trinidad Pier Replacement B-4 Boring Log
4. 2013 California Building Code Table 1806.2
5. USGS Seismic Hazard Data

Introduction:

Trinity Valley Consulting Engineers, Inc. (TVCE) was secured by the Trinidad Rancheria Transportation Department to evaluate the potential for a new structure that will accommodate the Trinidad Pier Interpretative Center. The center will contain an interpretative viewing area, bait shop, and associated administrative offices at 1 Bay Street, Trinidad, California. The following is an outline of our findings and recommendations.

Project Site Location:

The project site is a portion of Section 26, of Township 8 North, Range 1 East of H.B. & M in the city of Trinidad, in the County of Humboldt, State of California (see Attachment 1 for a Location Map). The Assessor Parcel Number (APN) for the property is 042-071-012. (see Attachment 2 for Assessor Map).

The parcel centroid is Latitude N 40.0554° (41°03'19.44"N) and Longitude W – 124.147° (124°08'49.20"W). The approximate site elevation is twenty three feet (23') above mean sea level according to Google Earth.

Project Site Conditions:

The subject property is currently occupied by the Trinidad Pier and associated facilities. Improvements include a new structure with exterior deck and an accessible ADA path to the pier area. Water services will be provided by City of Trinidad. Electric services will be provided by PG&E. Propane gas will be provided by Campora or Amerigas.

The adjoining features are the Trinidad Pier, Seascape Restaurant, Public Restrooms, Historic Lighthouse, and Trinidad State Beach.

Proposed Project:

The proposed project for this site is the construction of a new interpretative center that will accommodate a bait shop and administrative offices.

Grading for this site will be minimal and consist of excavation for the structure foundation. Total anticipated excavation will not exceed three feet in depth.

Site Soil Conditions:

Site Soil Conditions were interpolated from test boring logs generated by Taber Consultants for the Trinidad Pier Replacement project. The purpose of the investigation that occurred on July 2007 was to determine subsurface conditions subjacent of the former pier structure.

Information from the B-4 boring log closest to shore and approximately less than 100-feet from the proposed structure will be utilized for determining site soil conditions.

Recent marine deposits that mantle (~7 ft in thickness) Cretaceous/Jurassic-age Franciscan Formation (weathered to decomposed mudstone, shale, and sandstone) were discovered during the investigation. This thin veneer of marine sediments are comprised of loose to compact gray sand with shell fragments and gravel-sized fragments derived from the adjacent Trinidad Head and Little Head. Subjacent to the sandy materials are very soft gray and green clay, potentially a decomposed product of the underlying claystone and shale.

Site Soil Evaluation:

Site soils by conservative evaluation, will yield a bearing pressure of two thousand (2,000) psf for vertical bearing and one hundred and fifty (150) psf for lateral bearing

for the buildable areas (2019 California Building Code, Table 1806.2, see Attachment 4).

Total settlement is anticipated to be less than one inch (1"), and anticipated differential settlement will be less than three-quarters (¾) inch.

Seismic Considerations:

The project site is situated within a seismically active area, adjacent to several seismic sources capable of generating moderate to strong ground motions. As a result of being close to significant active faults (Trinidad fault to east of the site, the Mad River fault zone to the southeast, and Cascadia subduction zone offshore to the west), and several other faults both on- and off-shore, the project site is expected to experience strong ground-motion in the event a nearby earthquake during the design life of the structure.

The project site lies within one (1) mile of the active trace of the Trinidad Fault, which is a Type A fault. The project site does not lie within an Alquist-Priolo zone.

The project site is located in an area rated with moderate Instability, for seismic safety according to the County of Humboldt Web GIS mapping.

The following coefficients shall be used for seismic design:

Site Class	D
Mapped Spectral Response Acceleration (short), S_S:	2.384 g
Mapped Spectral Response Acceleration (1-sec), S_1:	0.989 g
Site Coefficient, Fa:	1.0
Site Coefficient, Fv:	1.3
Acceleration Spectral Response (short), S_{DS}:	1.589 g
Acceleration Spectral Response (1-sec), S_{D1}:	0.857 g
Seismic Design Category:	D
Occupancy Category:	II
Importance Factor:	1.0

Due to the site soils, depth to groundwater, and distance to the nearest known quaternary fault, the potential for surface rupture, liquefaction, soil strength loss and faulting at this site is Low.

Tsunami Considerations:

The project site has been mapped by the State of California as coordinated by Cal EMA to be within a Tsunami Hazard zone. It is typically understood by residents and developers with in these zones accept the risk of a tsunami event. Construction design should, where feasible, allow for the ground floor walls to be able to "blown out" in case of a tsunami to increase the survivability of the proposed structure.

Flood Considerations:

The project and vicinities have not been mapped by FEMA (FIRM Panel Unmapped_060436). However, the project site is located within the vicinity of Trinidad Bay and has a history of minor flooding, primarily associated with winter storms with strong onshore winds, heavy precipitation and high tides.

Conclusion:

No subsurface or seismic conditions were encountered which would prohibit the construction of the proposed single-story structure.

Recommendations:

Site Preparation:

Notify Underground Service Alert (1-800-227-2600) prior to any ground disturbing activities.

Strip and remove all topsoil and vegetation from the project area and for a minimum of three (3) feet to the outside of the proposed building footprint.

Scarify and compact the upper six (6) inches of soils for all areas to receive structural fills or other improvements

Foundation:

Foundations shall be either a raised perimeter, slab on grade, or isolated pier foundation, and shall be designed to provide bracing for all vertical and lateral structural loadings.

Slabs on grade shall have the following minimum requirements:

- 4" Concrete with reinforcement
- 2" Sand
- 6 mil vapor barrier
- 6" Crushed rock

Foundations shall not bear on sands existing at the site location. Alternatively, foundations should bear on subjacent clays or weathered bedrock to reduce risk of soil strength loss and liquefaction potential. Should additional topsoil or fill material be encountered, the foundation shall be extended down to the depth of native material. This can be accomplished by excavating trenches and backfilling to the bottom of the footings with concrete slurry or compacted base rock.

Foundation shall be constructed a minimum of ten (10) feet from the top of fill slopes and five feet from the toe of the cut slope.

All foundations shall be constructed in accordance with the 2019 California Building Code (CBC).

Grading:

All cut and fill slopes shall be 1-1/2:1 or flatter.

All fill material shall be placed in lifts not to exceed nine (9) inches in depth and should be compacted to a minimum of ninety percent (90%) relative compaction per ASTM D1557.

Finished grading should provide a minimum slope of two percent away from buildings and foundations for a minimum of ten linear feet.

Erosion Control:
Use Best Management Practices (BMPs) in order to minimize sediment transport offsite. The following should be implemented for the site at a minimum:

EC-1: Scheduling of construction and BMP implementation.
EC-2: Preservation of Existing Vegetation.
NS-8: Vehicle and equipment cleaning practices.
NS-9: Vehicle and equipment fueling practices.
NS-10: Vehicle and equipment maintenance practices.
NS-12: Concrete curing practices.
WM-1: Material delivery and storage practices.
WM-3: Stockpile management.
WM-4: Spill prevention and control.
WM-5: Solid waste management.
WM-6: Hazardous waste management.
WM-8: Concrete waste management.

References:

California Building Code – Volume 2, 2019

Geologic Map of California
California Division of Mines and Geology

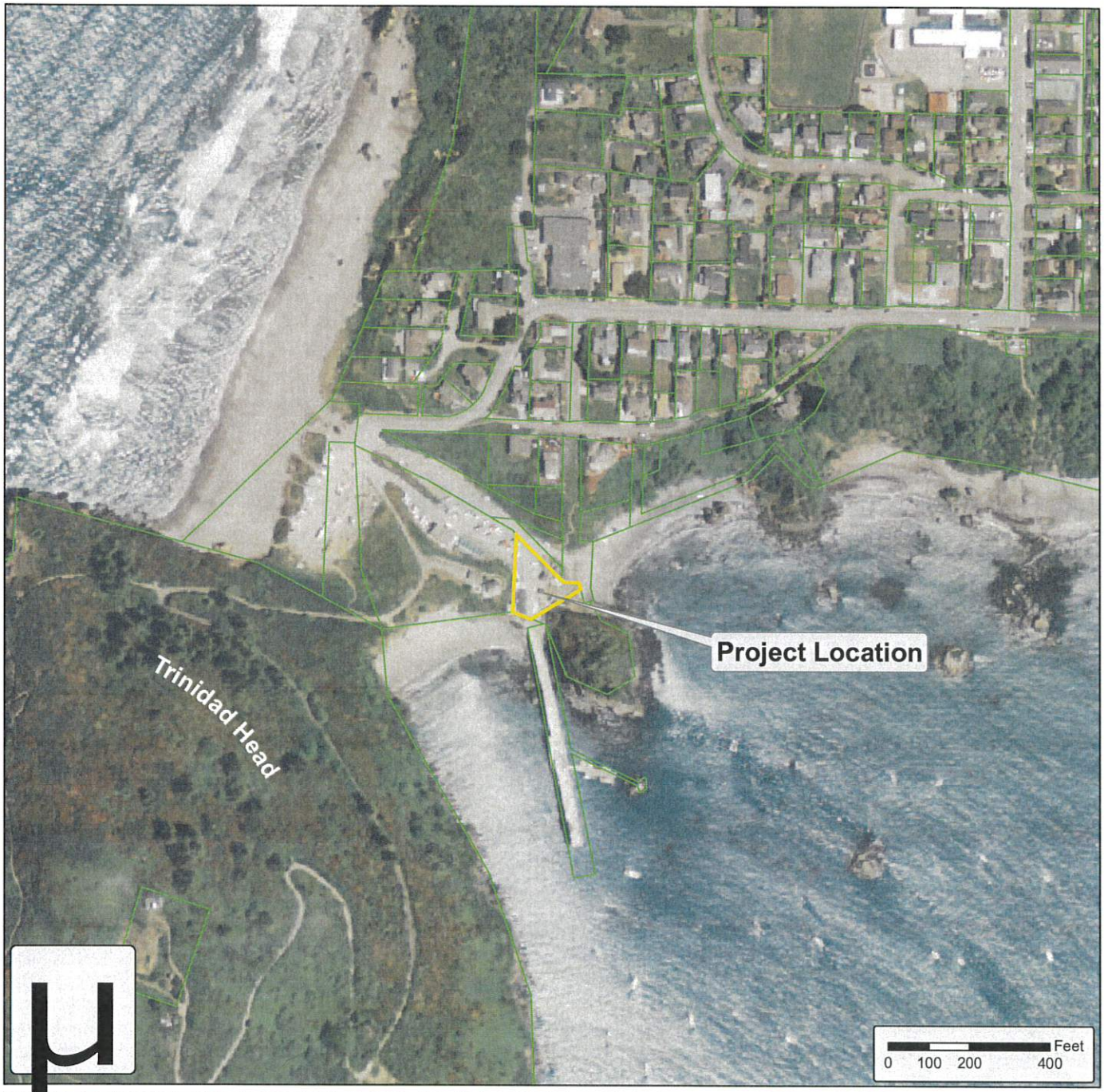
Google Earth

US Geologic Survey Hazard Mapping

County of Humboldt GIS Mapping

Attachment 1:

Location Map



TVCE
Trinity Valley Consulting Engineers Inc

6/18/2014 - CXF
TVCE Job# 605

Aerial Imagery:

2012 USDA NAIP Imagery

(http://datagateway.nrcs.usda.gov/)

Project: R2 Soils Report
Trinidad Rancheria
Interpretative Center
APN: 042-071-012

Attachment 1 - Location Map

Attachment 2:

Assessor Parcel Map

HUMBOLDT COUNTY
GEOGRAPHIC INFORMATION SYSTEMS
ONLINE MAPPING

Humboldt County Web GIS Map

Prosperity!

Legend

Coastal and Dam Inundation
- Dams
- Dam Failure Inundation
- Levees

Coastal Erosion Vulnerability Index
- Low
- Moderate
- High
- Very High

Seismic Safety and Slope
- Area of Potential Liquefaction

Seismic Safety

Slope Stability
- 3 High Instability
- 2 Moderate Instability
- 1 Low Instability
- 0 Relatively Stable
- Historic Landslides

Slope USGS

Percent Slope
- 15-30%
- 30 - 50%
- +50%

Earthquake
- Alquist Priolo Fault
- Alquist Priolo Zone

NEHRP Soils - HAZUS

Geologic Units
- B - Plutonic/Metamorphic
- BC - Franciscan complex
- C - Franciscan melange/serp
- CD - Sedimentary
- D - Younger alluvium

Flood

FEMA Flood Zones
- A - 100 Year
- B - 500 Year
- Floodway

DWR Awareness Floodplain
- Awareness Floodplain

Fire

Fire Response
- District
- District Response Area
- Non-Dist Response Area

Fire Rating (County)
- Extreme
- High
- Moderate
- Low
- Nil

Fire Hazard Severity (CALFIRE)
- Very High
- High
- Moderate

Critical Facilities

Hospitals and Clinics
- HOSPITAL
- CLINIC
- RED CROSS
- PHARMACY

Emergency Response
- FIRE DEPARTMENT
- VOLUNTEER FIRE DEPT
- FOREST FIRE STATION
- SHERIFF
- CHP
- POLICE DEPARTMENT
- AMBULANCE
- AIR ATTACK BASE (AAB)

Airports
- AIRPORT - COMMERCIAL
- AIRPORT; SECONDARY
- HELICOPTER

Misc Critical Facilities
- CDF HEADQUARTERS
- RANGER STATION
- CAMPGROUND
- OTHER CRITICAL FACILITIES

General Map Themes
- Parcels

Highways and Roads

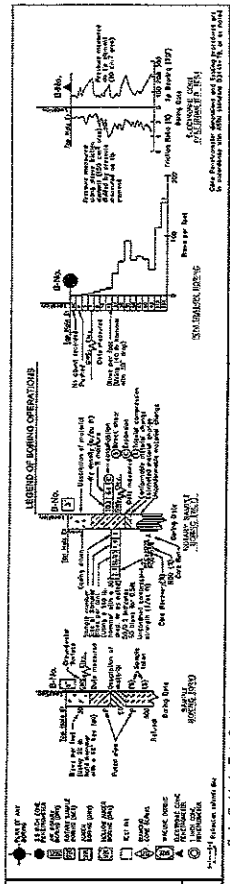
Functional Class
- Principal Arterials
- Minor Arterials
- Major Collectors
- Minor Collectors
- Local Roads
- Private or Unclassified
- City Boundary

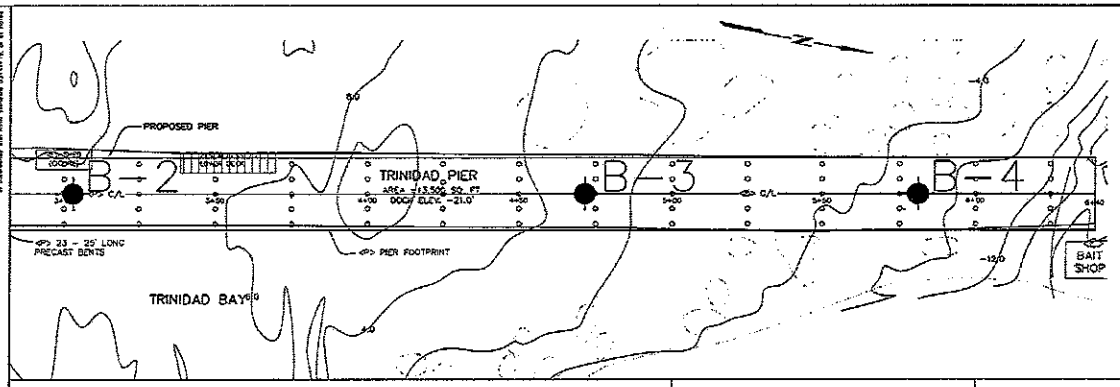
Freeance PDF Printing System
Humboldt County Web GIS
http://gis.co.humboldt.ca.us

Map Disclaimer: While every effort has been made to assure the accuracy of this information, it should be understood that it does not have the force and effect of law, rule, or regulation. Should any difference or error occur, the law will take precedence. Humboldt County assumes no liability or responsibility in the use, or misuse, of this data.

Attachment 3:

Taber Consultants Trinidad Pier Replacement B-4 Boring Log





REFERENCE:
Site layout and elevations provided by Pacific Affiliates Inc.

DIST.	COUNTY	ROUTE	POST MILES TOTAL PROJECT	SHEET NO.	TOTAL SHEETS
	Hum			2	2

REGISTERED ENGINEERING GEOLOGIST

REGISTERED GEOLOGIST DAVID A. KITTLEMAN No. 2412 EXP. 12-31-07 CERTIFIED ENGINEERING GEOLOGIST STATE OF CALIFORNIA

PLANS APPROVAL DATE

TABER CONSULTANTS
3911 West Capitol Avenue
West Sacramento, CA 95691-2116

JOB No. 1P2/507/089 LOCATION: 41124-AZ: 580N: 560W

NOTES:
1. Field classification of soils was in accordance with ASTM D 2488-00 "Description and Identification of Soils (Visual-Manual Procedure)".

2. Rock classification according to Bureau of Reclamation, U.S. Department of the Interior, "Engineering Geology Field Manual", and "Manual of Field Geology", (Compton, Robert R.).

3. Standard Penetration tests were performed in accordance with ASTM D 1586-99 using a hammer operated with an automated drop system. Drill rods were 1 5/8-inch diameter "A"-rods; sampler was driven with brass liners.

4. The length of each sampled interval is shown graphically on the boring log. Whole number blow counts ("N") represent the "standard penetration resistance" used in accordance with ASTM D1586-99. Where less than 1 foot of penetration is achieved, the blow count shown is for that fraction of the "standard penetration resistance" interval actually penetrated.

5. Consistency of soils shown in ( ) where estimated.

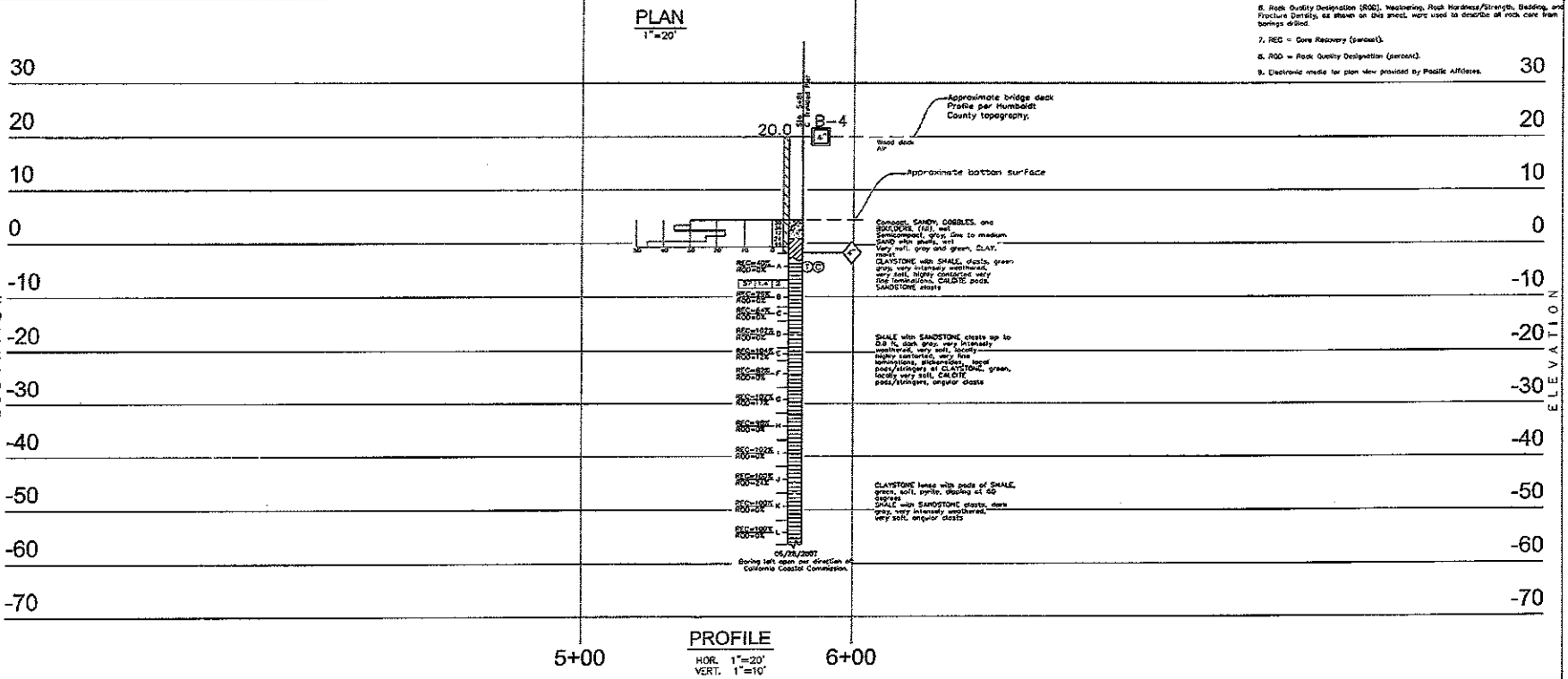
6. Rock Quality Designation (RQD), Weathering, Rock Hardness/Strength, Bedding, and Fracture Details, as shown on this sheet, were used to describe all rock core from borings drilled.

7. REC = Core Recovery (percent).

8. RQD = Rock Quality Designation (percent).

9. Electronic media for plan view provided by Pacific Affiliates.

PLAN
1"=20'



DESIGN OVERSIGHT	DRAWN BY	M.Lattin	E.A. Hilmer FIELD INVESTIGATOR	PREPARED FOR Cher-Ae Indian Community of the Trinidad Rancheria	BRIDGE NO.
SIGN OFF DATE	CHECKED BY	M.Kellogg	DATE July 2007	PROJECT ENGINEER	POST MILE

TRINIDAD PIER REPLACEMENT

LOG OF TEST BORINGS (2 OF 2)

08/17/2007 1P2507089.dwg

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS 0 1 2 3

CU
SA

REVISED PRINTS BEARING EARLIER REVISION DATES

REVISION DATES (PRELIMINARY STAGE ONLY)

SHEET OF 2 2

Attachment 4:

2019 California Building Code

Table 1806.2

TABLE 1806.2
PRESUMPTIVE LOAD-BEARING VALUES

CLASS OF MATERIALS	VERTICAL FOUNDATION PRESSURE (psf)	LATERAL BEARING PRESSURE (psf/ft below natural grade)	LATERAL SLIDING RESISTANCE	
			Coefficient of friction[a]	Cohesion (psf)[b]
1. Crystalline bedrock	12,000	1,200	0.70	—
2. Sedimentary and foliated rock	4,000	400	0.35	—
3. Sandy gravel and/or gravel (GW and GP)	3,000	200	0.35	—
4. Sand, silty sand, clayey sand, silty gravel and clayey gravel (SW, SP, SM, SC, GM and GC)	2,000	150	0.25	—
5. Clay, sandy clay, silty clay, clayey silt, silt and sandy silt (CL, ML, MH and CH)	1,500	100	—	130

For SI: 1 pound per square foot = 0.0479 kPa, 1 pound per square foot per foot = 0.157 kPa/m.

a. Coefficient to be multiplied by the dead load.

b. Cohesion value to be multiplied by the contact area, as limited by Section 1806.3.2.

Attachment 5:

United States Geological Survey
Seismic Hazard Data

USGS Design Maps Detailed Report

ASCE 7-10 Standard (41.05667°N, 124.14722°W)

Site Class C – "Very Dense Soil and Soft Rock", Risk Category I/II/III

Section 11.4.1 — Mapped Acceleration Parameters

Note: Ground motion values provided below are for the direction of maximum horizontal spectral response acceleration. They have been converted from corresponding geometric mean ground motions computed by the USGS by applying factors of 1.1 (to obtain S_S) and 1.3 (to obtain S_1). Maps in the 2010 ASCE-7 Standard are provided for Site Class B. Adjustments for other Site Classes are made, as needed, in Section 11.4.3.

From Figure 22-1 [1] $S_S = 2.384$ g

From Figure 22-2 [2] $S_1 = 0.989$ g

Section 11.4.2 — Site Class

The authority having jurisdiction (not the USGS), site-specific geotechnical data, and/or the default has classified the site as Site Class C, based on the site soil properties in accordance with Chapter 20.

Table 20.3–1 Site Classification

Site Class	$\bar{v}_s$	$\bar{N}$ or $\bar{N}_{ch}$	$\bar{s}_u$
A. Hard Rock	>5,000 ft/s	N/A	N/A
B. Rock	2,500 to 5,000 ft/s	N/A	N/A
C. Very dense soil and soft rock	1,200 to 2,500 ft/s	>50	>2,000 psf
D. Stiff Soil	600 to 1,200 ft/s	15 to 50	1,000 to 2,000 psf
E. Soft clay soil	<600 ft/s	<15	<1,000 psf
	Any profile with more than 10 ft of soil having the characteristics: • Plasticity index PI > 20, • Moisture content $w \geq 40\%$, and • Undrained shear strength $\bar{s}_u$ < 500 psf		
F. Soils requiring site response analysis in accordance with Section 21.1	See Section 20.3.1		

For SI: 1ft/s = 0.3048 m/s 1lb/ft² = 0.0479 kN/m²

Section 11.4.3 — Site Coefficients and Risk-Targeted Maximum Considered Earthquake (MCE_R) Spectral Response Acceleration Parameters

Table 11.4-1: Site Coefficient F_a

Site Class	Mapped MCE_R Spectral Response Acceleration Parameter at Short Period				
	$S_S \le 0.25$	$S_S = 0.50$	$S_S = 0.75$	$S_S = 1.00$	$S_S \ge 1.25$
A	0.8	0.8	0.8	0.8	0.8
B	1.0	1.0	1.0	1.0	1.0
C	1.2	1.2	1.1	1.0	1.0
D	1.6	1.4	1.2	1.1	1.0
E	2.5	1.7	1.2	0.9	0.9
F	See Section 11.4.7 of ASCE 7				

Note: Use straight-line interpolation for intermediate values of S_S

For Site Class = C and S_s = 2.384 g, F_a = 1.000

Table 11.4-2: Site Coefficient F_v

Site Class	Mapped MCE_R Spectral Response Acceleration Parameter at 1-s Period				
	$S_1 \le 0.10$	$S_1 = 0.20$	$S_1 = 0.30$	$S_1 = 0.40$	$S_1 \ge 0.50$
A	0.8	0.8	0.8	0.8	0.8
B	1.0	1.0	1.0	1.0	1.0
C	1.7	1.6	1.5	1.4	1.3
D	2.4	2.0	1.8	1.6	1.5
E	3.5	3.2	2.8	2.4	2.4
F	See Section 11.4.7 of ASCE 7				

Note: Use straight-line interpolation for intermediate values of S_1

For Site Class = C and S_1 = 0.989 g, F_v = 1.300

Equation (11.4-1): $S_{MS} = F_a S_S = 1.000 \times 2.384 = 2.384$ g

Equation (11.4-2): $S_{M1} = F_v S_1 = 1.300 \times 0.989 = 1.285$ g

Section 11.4.4 — Design Spectral Acceleration Parameters

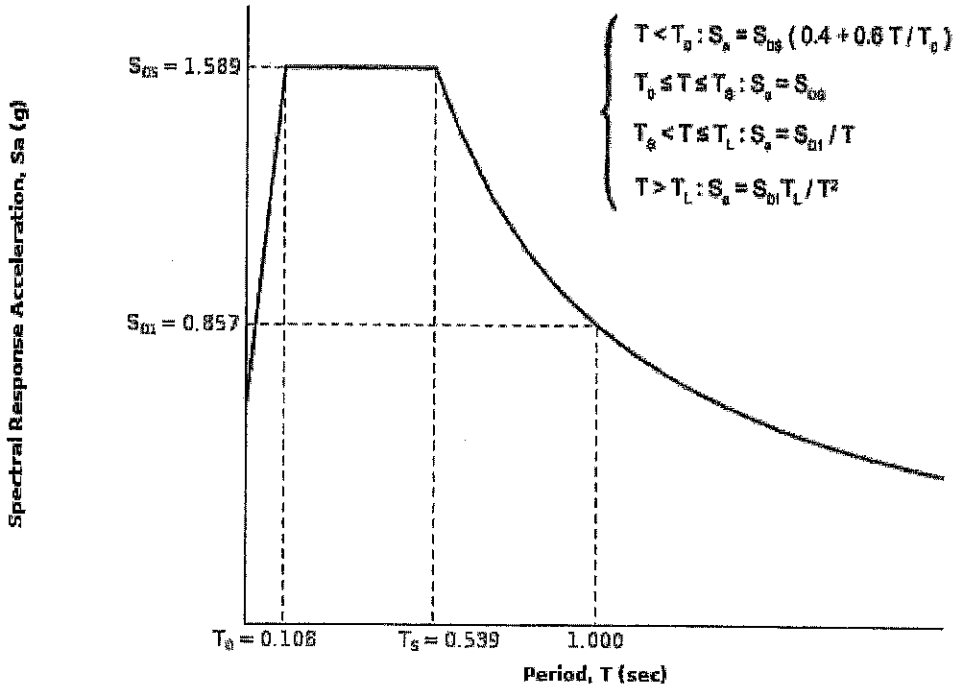
Equation (11.4-3): $S_{DS} = \frac{2}{3} S_{MS} = \frac{2}{3} \times 2.384 = 1.589$ g

Equation (11.4-4): $S_{D1} = \frac{2}{3} S_{M1} = \frac{2}{3} \times 1.285 = 0.857$ g

Section 11.4.5 — Design Response Spectrum

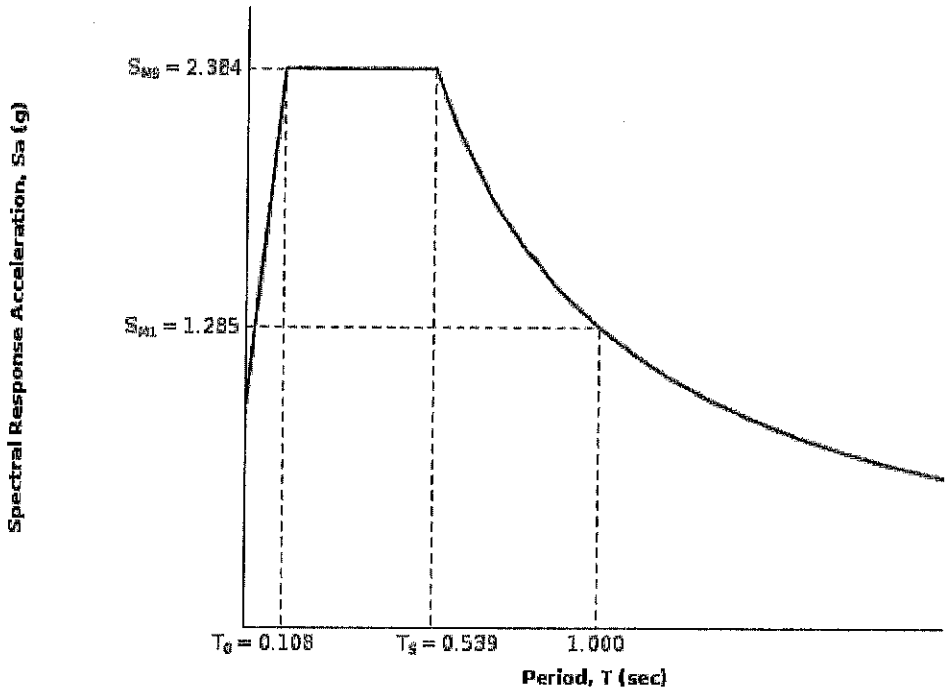
From Figure 22-12 [3] $T_L = 8$ seconds

Figure 11.4-1: Design Response Spectrum



Section 11.4.6 — Risk-Targeted Maximum Considered Earthquake (MCE_R) Response Spectrum

The MCE_R Response Spectrum is determined by multiplying the design response spectrum above by 1.5.



Section 11.6 — Seismic Design Category

Table 11.6-1 Seismic Design Category Based on Short Period Response Acceleration Parameter

VALUE OF S_{DS}	RISK CATEGORY		
	I or II	III	IV
$S_{DS} < 0.167g$	A	A	A
$0.167g \le S_{DS} < 0.33g$	B	B	C
$0.33g \le S_{DS} < 0.50g$	C	C	D
$0.50g \le S_{DS}$	D	D	D

For Risk Category = I and S_{DS} = 1.589 g, Seismic Design Category = D

Table 11.6-2 Seismic Design Category Based on 1-S Period Response Acceleration Parameter

VALUE OF S_{D1}	RISK CATEGORY		
	I or II	III	IV
$S_{D1} < 0.067g$	A	A	A
$0.067g \le S_{D1} < 0.133g$	B	B	C
$0.133g \le S_{D1} < 0.20g$	C	C	D
$0.20g \le S_{D1}$	D	D	D

For Risk Category = I and S_{D1} = 0.857 g, Seismic Design Category = D

Note: When S_1 is greater than or equal to 0.75g, the Seismic Design Category is **E** for buildings in Risk Categories I, II, and III, and **F** for those in Risk Category IV, irrespective of the above.

Seismic Design Category ≡ "the more severe design category in accordance with Table 11.6-1 or 11.6-2" = E

Note: See Section 11.6 for alternative approaches to calculating Seismic Design Category.

References

1. *Figure 22-1*: http://earthquake.usgs.gov/hazards/designmaps/downloads/pdfs/2010_ASCE-7_Figure_22-1.pdf
2. *Figure 22-2*: http://earthquake.usgs.gov/hazards/designmaps/downloads/pdfs/2010_ASCE-7_Figure_22-2.pdf
3. *Figure 22-12*: http://earthquake.usgs.gov/hazards/designmaps/downloads/pdfs/2010_ASCE-7_Figure_22-12.pdf
4. *Figure 22-7*: http://earthquake.usgs.gov/hazards/designmaps/downloads/pdfs/2010_ASCE-7_Figure_22-7.pdf
5. *Figure 22-17*: http://earthquake.usgs.gov/hazards/designmaps/downloads/pdfs/2010_ASCE-7_Figure_22-17.pdf
6. *Figure 22-18*: http://earthquake.usgs.gov/hazards/designmaps/downloads/pdfs/2010_ASCE-7_Figure_22-18.pdf